

Geometry Independent Field approximaTion error-driven local adaptivity in elasto-dynamics

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Outline



- > Motivation
- > Problem statement
- > Methodology
- > Numerical examples
- > Conclusions and Following work



- NURBS-IGA performs better than FEM in dynamics. (High order continuity)
- IGA--- IsoGeometric Analysis

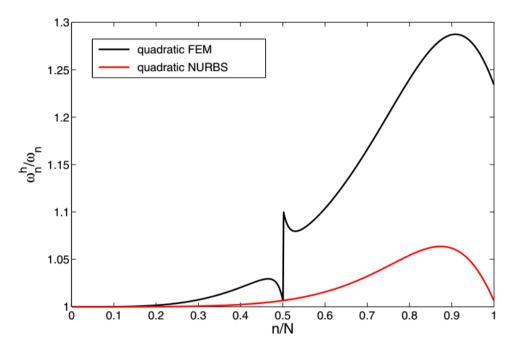
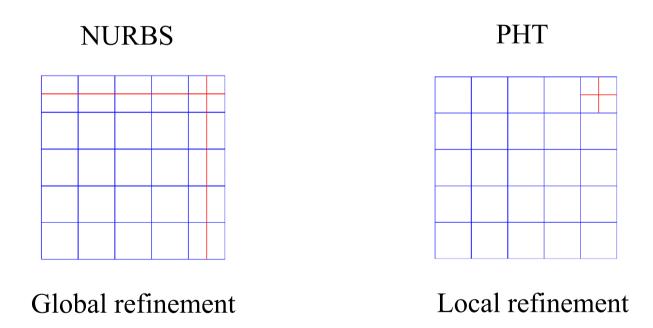


Fig. 8. Fixed-fixed rod. Normalized discrete spectra using quadratic finite elements and NURBS.

Cottrell, J. Austin, et al. "Isogeometric analysis of structural vibrations." *Computer methods in applied mechanics and engineering* 195.41 (2006): 5257-5296.



- NURBS are limited to global refinement. (Tensor product)
- PHT are with local refinement. (Hierarchical)
- PHT --- Polynomial splines over Hierarchical T-meshes



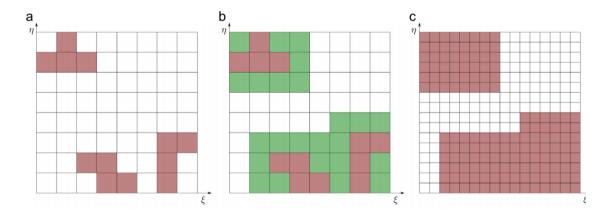
 But PHT will lose information of geometry for curves because it is not rational



- RHT can describe curves but limited to $\,C^1_{\,ullet}$
- RHT --- Rational splines over Hierarchical T-meshes

Nguyen-Thanh, N., et al. "An adaptive three-dimensional RHT-splines formulation in linear elasto-statics and elasto-dynamics." *Computational Mechanics* 53.2 (2014): 369-385.

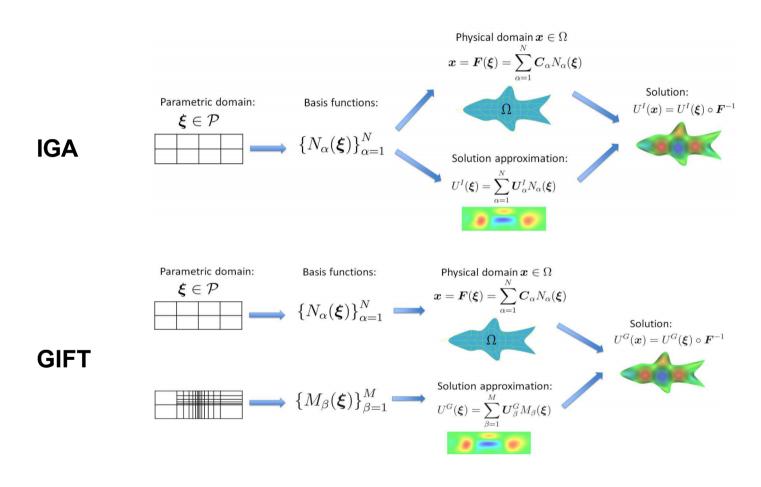
- Multiple-patch NURBS can achieve local refinement
- But over-accuracy and lose continuity at boundary of patch



Chemin, Alexandre, Thomas Elguedj, and Anthony Gravouil. "Isogeometric local h-refinement strategy based on multigrids." Finite Elements in Analysis and Design 100 (2015): 77-90.



GIFT--- Geometry-Independent Field approximaTion



GIFT: NURBS + PHT

exact geometry local refinement

Problem statement



Variational equation for vibration problem

$$\int_{\Omega} \boldsymbol{\varepsilon}^{T}(\boldsymbol{\delta}\mathbf{u}) \mathbf{D} \boldsymbol{\varepsilon}(\mathbf{u}) d\Omega + \int_{\Omega} \rho \delta \mathbf{u}^{T} \ddot{\mathbf{u}} d\Omega = 0, \ \forall \delta \mathbf{u} \in \mathscr{V}$$

Kirchoff plate theory (thin plate)

$$\mathbf{u} = \{\boldsymbol{u}, \boldsymbol{v}, \boldsymbol{w}\}^T = \left\{-z\frac{\partial}{\partial x}, -z\frac{\partial}{\partial y}, 1\right\}^T \boldsymbol{w} = \mathbb{H}w$$

$$\boldsymbol{\varepsilon}(\mathbf{u}) = \left\{ \frac{\partial^2}{\partial x^2}, \frac{\partial^2}{\partial y^2}, 2 \frac{\partial^2}{\partial x \partial y} \right\}^T \boldsymbol{w} = \mathbb{E} \boldsymbol{w}$$

w is independent variable

$$\int_{\Omega} \mathbb{E} \delta \boldsymbol{w}^T \mathbf{D} \mathbb{E} \boldsymbol{w} d\Omega + \int_{\Omega} \rho \mathbb{H} \delta \boldsymbol{w}^T \mathbb{H} \ddot{\boldsymbol{w}} d\Omega = 0, \ \forall \delta \boldsymbol{w} \in \mathscr{V}$$

Problem statement



NURBS

GIFT form of discrete governing equation

$$\mathbf{K} = \int_{\mathcal{P}} \mathbb{E} \boldsymbol{M}^{T}(\boldsymbol{\xi}) \mathbf{D} \mathbb{E} \boldsymbol{M}(\boldsymbol{\xi}) | \mathbf{J}(\boldsymbol{\xi})| d\boldsymbol{\xi}$$

$$\mathbf{M} = \int_{\mathcal{P}} \rho \mathbb{H} \boldsymbol{M}^{T}(\boldsymbol{\xi}) \mathbb{H} \boldsymbol{M}(\boldsymbol{\xi}) | \mathbf{J}(\boldsymbol{\xi})| d\boldsymbol{\xi}$$

$$\mathbf{J}(\boldsymbol{\xi}) = \frac{\partial \boldsymbol{x}}{\partial \boldsymbol{\xi}} = \sum_{i=1}^{n} \frac{\partial N_{i}(\boldsymbol{\xi})}{\partial \boldsymbol{\xi}} \boldsymbol{P}_{i}$$
PHT

$$\mathbf{J}(\boldsymbol{\xi}) = rac{\partial oldsymbol{x}}{\partial oldsymbol{\xi}} = \sum_{i=1}^n rac{\partial N_i(oldsymbol{\xi})}{\partial oldsymbol{\xi}} oldsymbol{P}_i$$

$$\mathbf{K}\boldsymbol{w} + \mathbf{M}\ddot{\boldsymbol{w}} = 0$$

Separation of solution for static state

$$\boldsymbol{w}(t) = \boldsymbol{\phi} \exp(i\lambda t)$$

Eigenvalue problem

$$(\mathbf{K} - \lambda^2 \mathbf{M}) \phi = 0$$

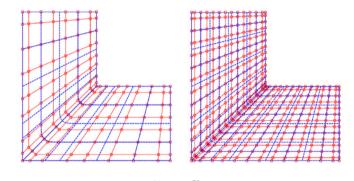
 $\lambda \rightarrow \text{Natural frequency} \qquad \phi \rightarrow \text{Eigenvector}$



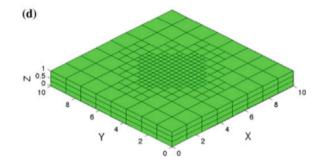
Refinement by IGA for vibration

Shojaee, S., et al. "Free vibration analysis of thin plates by using a NURBS-based isogeometric approach." *Finite Elements in Analysis and Design* 61 (2012): 23-34.

Nguyen-Thanh, N., et al. "An adaptive three-dimensional RHT-splines formulation in linear elastostatics and elasto-dynamics." *Computational Mechanics* 53.2 (2014): 369-385.



NURBS refinement (over-accurate and waste CPU time)



RHT prior refinement (not practical)



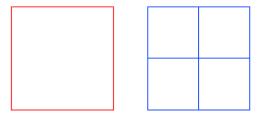
Error-driven adaptivity for vibration

Error estimation (not limited to vibration)

Error indicator $e = \tilde{\mathbf{u}} - \mathbf{u}^h$

$$\tilde{\mathbf{u}} \rightarrow \text{Solution in coarse mesh}$$

$$\tilde{\mathbf{u}} \rightarrow \text{Solution in coarse mesh}$$
 $\mathbf{u}^h \rightarrow \text{Solution in reference mesh}$



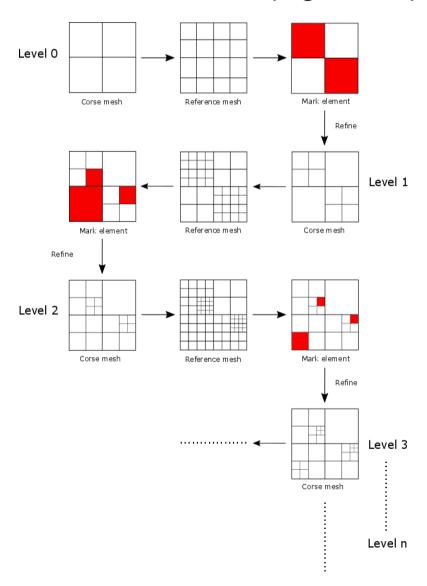
L2 norm of error indicator for element

$$\|\boldsymbol{e}\|_{\Omega_e^i} = \left[\int_{\Omega_e^i} (\tilde{\mathbf{u}} - \mathbf{u}^h)^T (\tilde{\mathbf{u}} - \mathbf{u}^h) d\Omega_e^i \right]^{\frac{1}{2}}, \ \bigcup_{i=1}^N \Omega_e^i = \Omega$$

If
$$\|oldsymbol{e}\|_{\Omega^i_e}>arepsilon_e$$
 tolerance, refine element i



Adaptive hierarchical local refinement (Algorithm 1)





Error-driven local adaptivity for vibration

Error indicator for natural frequency

$$e_{\lambda}^{i} = \left| \frac{\tilde{\lambda}^{\tilde{i}} - \lambda^{i}}{\tilde{\lambda}^{\tilde{i}}} \right|$$

 $\tilde{\lambda}^{\tilde{i}} \rightarrow \tilde{i}$ th natural frequency in reference mesh

 $\lambda^i \longrightarrow i$ th natural frequency in coarse mesh

If $e^i_{\lambda}>arepsilon_{\lambda}$, define error indicator for normalized eigenvector

$$oldsymbol{e}_{oldsymbol{\phi}}^{i} = ilde{oldsymbol{\phi}}_{N, ilde{i}} - oldsymbol{\phi}_{N,i}^{h}$$

• Call **Algorithm 1** $(\tilde{\mathbf{u}}=\tilde{\boldsymbol{\phi}}_{N,\tilde{i}},\mathbf{u}^h=\boldsymbol{\phi}_{N,i}^h)$ to process local refinement at mode i



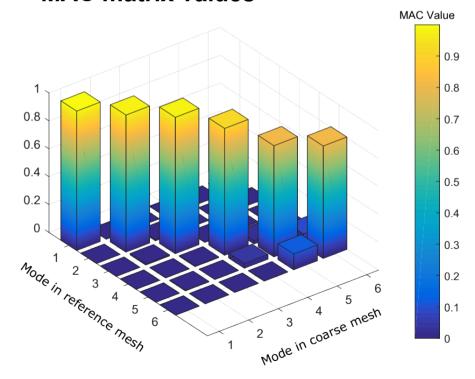


Modal Assurance Criterion (MAC)

$$\mathcal{M}(\tilde{i},i)|_{\tilde{\boldsymbol{\phi}}_{N,\tilde{i}},\boldsymbol{\phi}_{N,i}^{h}} = \frac{(\tilde{\boldsymbol{\phi}}_{N,\tilde{i}}^{T} \cdot \boldsymbol{\phi}_{N,i}^{h})^{2}}{\|\tilde{\boldsymbol{\phi}}_{N,\tilde{i}}\|^{2}\|\boldsymbol{\phi}_{N,i}^{h}\|^{2}}$$

Consistent correspondence check between mode shapes

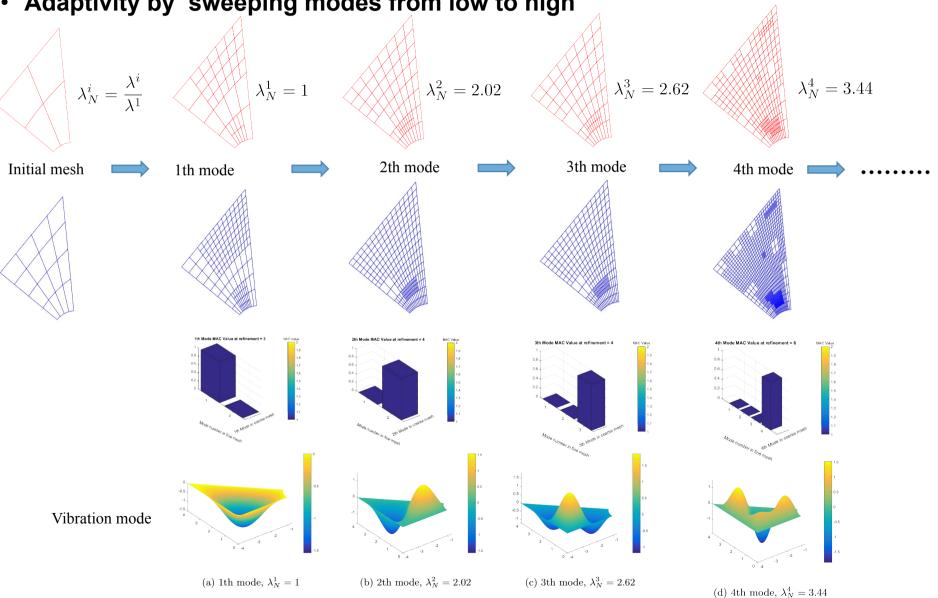
MAC matrix values



Values of MAC matrix \mathcal{M} (row is for \tilde{i} , column is for i)						
Mode	1	2	3	4	5	6
1	0.9995	0.0001	0.0003	0.0000	0.0000	0.0000
2	0.0001	0.9825	0.0087	0.0030	0.0018	0.0000
3	0.0003	0.0089	0.9777	0.0029	0.0024	0.0001
4	0.0000	0.0016	0.0052	0.9093	0.0233	0.0085
5	0.0000	0.0034	0.0008	0.0343	0.7970	0.0830
6	0.0000	0.0001	0.0013	0.0020	0.1133	0.8062



Adaptivity by sweeping modes from low to high





Heterogeneous platehole

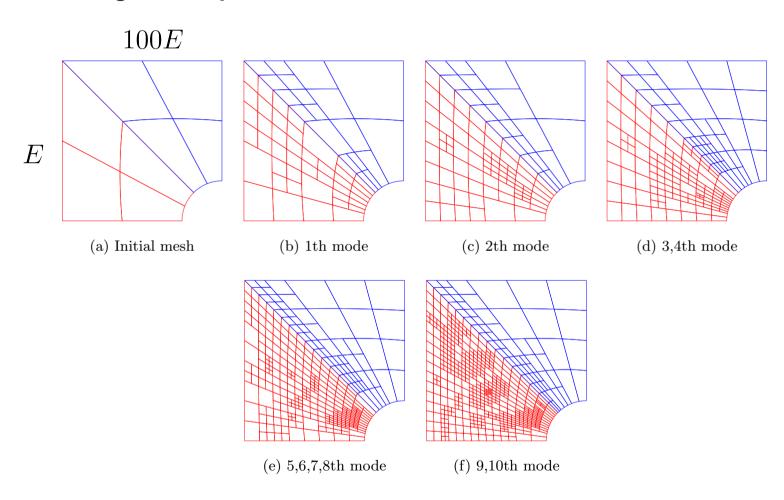
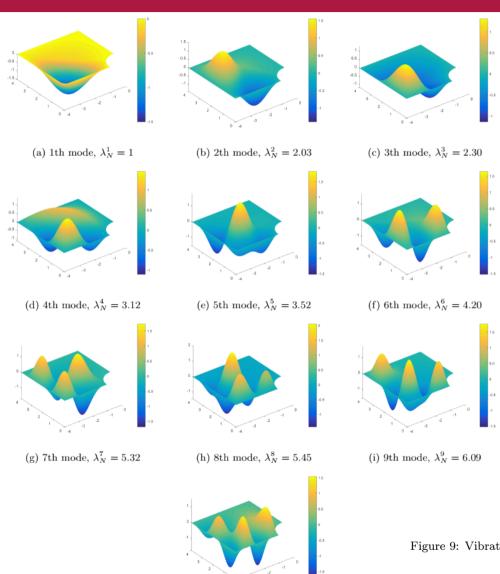


Figure 8: Adaptive refinement process of 1-10th mode for vibration of Kirchoff composite platehole ($E_2 = 100E_1$) in coarse mesh. Geometry-NURBS, Solution Field-PHT, p =3, q =3.





(j) 10th mode, $\lambda_N^{10} = 6.93$

Figure 9: Vibration at 1-10th mode of Kirchoff composite platehole ($E_2 = 100E_1$).



Heterogeneous Lshaped-bracket non-symmetric boundary condition

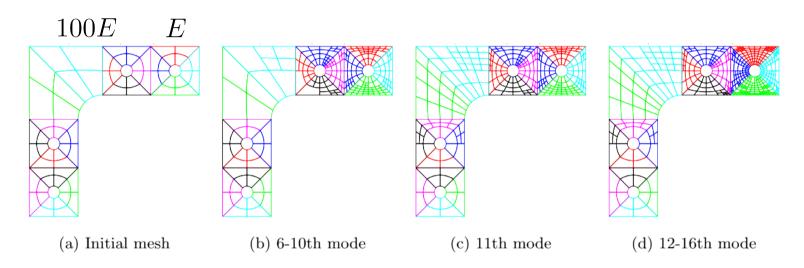
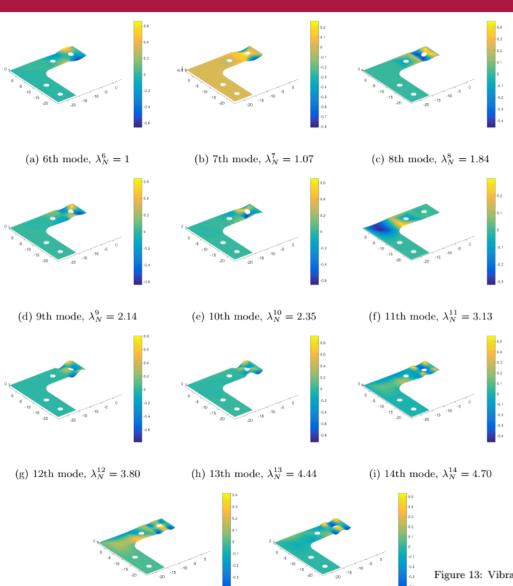


Figure 12: Adaptive refinement process of 6-16th mode for vibration of Kirchoff composite LShaped-bracket (right-up 4 patches are soft) in coarse mesh. Geometry-NURBS, Solution Field-PHT, p =3, q =3.





(k) 16th mode, $\lambda_N^{16} = 5.23$

(j) 15th mode, $\lambda_N^{15} = 4.72$

Figure 13: Vibration at 6-16th mode of Kirchoff composite LShaped-bracket (right-up 4 patches are soft).

Conclusions and Following work



- Contributions
 - I. GIFT for dynamics
 - II. Error-driven adaptive hierarchical local refinement
 - III. Modal analysis

- Following work
 - I. 3D vibration
 - II. Space-time adaptivity



Thank you!